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GEOTECHNICAL LOG OF NON-CORE DRILLHOLE

Borehole No: BH202

Sheet No: 1 OF 2

Project No: 3003659

Client: QDTMR

Project: Smith Olsen Detailed Design Geotech Investigation Co-ordinates System: UTM Zone 56



Feature: Easting: 534896.2m E

Location: Refer Location Plan Northing: 6906962.7m S

Surface RL (m): 41.45

Angle from Horz: 90

Direction: n/a

DRILLING							TESTING				SUBSTANCE						
Method	Support	Rate			Water	Sample	Depth (m)	Depth/RL	Type	Sample or Field Test	Graphic Log	USC Symbol	Description	Moisture	Consistency/ Density	Other Observations	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	
ATC	HW Casing.	---	---	---		SPT	41.45	S	30/10mm	H.B.			ASPHALT	D	D	SPT Hammer Bouncing	
							41.30						Silty GRAVEL: Dense, fine to medium gravel, grey, dry, road base.				
							0.60						METASILTSTONE: Extremely low strength, extremely weathered, yellow-orange.				
							40.85										
							1										
WD		---	---	---		SPT	1.30	S	H.B.			Very low strength to low strength, highly weathered, dark grey trace green-grey.			SPT Hammer Bouncing		
							40.15										
							2										
							2.50										
							3										
												Refer to Geotechnical Log of Cored Drillhole					

Notes (Instrumentation etc): Due to light post moved borehole 2.3m E, 4.0m offset on road shoulder

Contractor: GeoDrill

Commenced: 05/10/11

Logged By: ME/BD

Equipment: Hydropower Scout

Completed: 05/10/11

Checked By: AR

Basis of description and details of abbreviations are given on explanatory notes



GEOTECHNICAL INVESTIGATIONS LOG

Borehole No: BH202

Sheet No: 2 OF 2

Project No: 3003659

Client: QDTMR

Project: Smith Olsen Detailed Design Geotech Investigation Co-ordinates System: UTM Zone 56

Feature: E: 534896.2

Location: Refer Location Plan

N: 6906962.7

Surface RL (m): 41.45

Angle from Horiz: 90

Direction: n/a

DRILLING					SUBSTANCE										TEST		DEFECTS									
Method	Water	TCR %	RQD %	Lift	Depth (m)	Depth/RL	Graphic Log	Description ROCK TYPE, mineralogy, grain size, colour, fabric, etc.	Weathering					Estimated Strength					Type	Result	Fracture Spacing (mm)			Depth	Description Type, Orientation, Spacing, Infilling, Coating, Planarity, Roughness, Thickness.	
									EW	HW	MW	SW	FS	FR	EL	VL	L	M			H	VH	EH			20
1	2	3	4	5	6	7	8	9	10					11					12	13	14			15	16	
NMLC								Refer to Geotechnical log of Non-cored Drillhole																		
						2.50		Start Coring at 2.50m																		
			100	66		38.95		METASILTSTONE: Dark grey with pale grey laminations, some iron staining on laminations and healed fractures, fractured, medium strength, slightly weathered.																		
					3.3																					
			100	44																						
					4.2																					
			100	0			4.15		Dark grey, highly fractured.																	
					4.6		4.45		Iron stained, fragmented.																	
							36.90		Medium to high strength.																	
			100	30																						
					5.2																					
							5.20		Some pale grey laminations, high strength, slightly weathered to fresh.																	
							36.25																			
		100	76																							
				7.0		7.00		Some iron staining in joints, fresh.																		
						34.45																				
		100	66																							
				9.1				Hole discontinued at 9.10m																		

Notes (Instrumentation etc): Due to light post moved borehole 2.3m E, 4.0m offset on road shoulder

Contractor: GeoDrill

Commenced: 05/10/11

Logged By: ME/BD

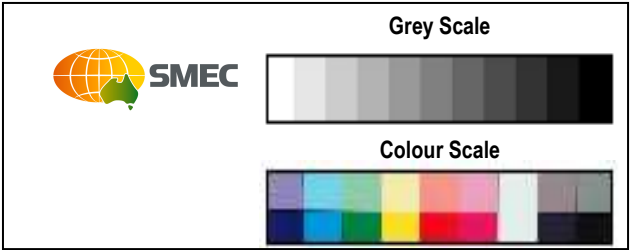
Equipment: Hydropower Scout

Completed: 05/10/11

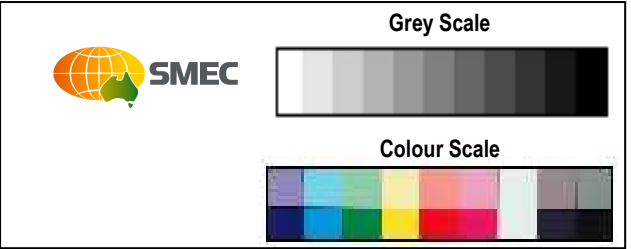
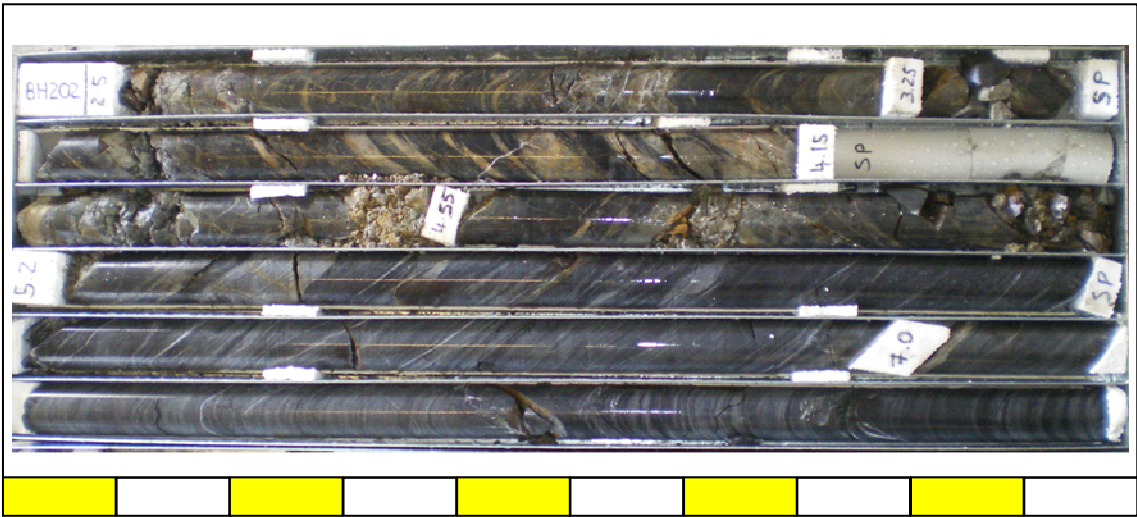
Checked By: AR

Basis of description and details of abbreviations are given on explanatory notes

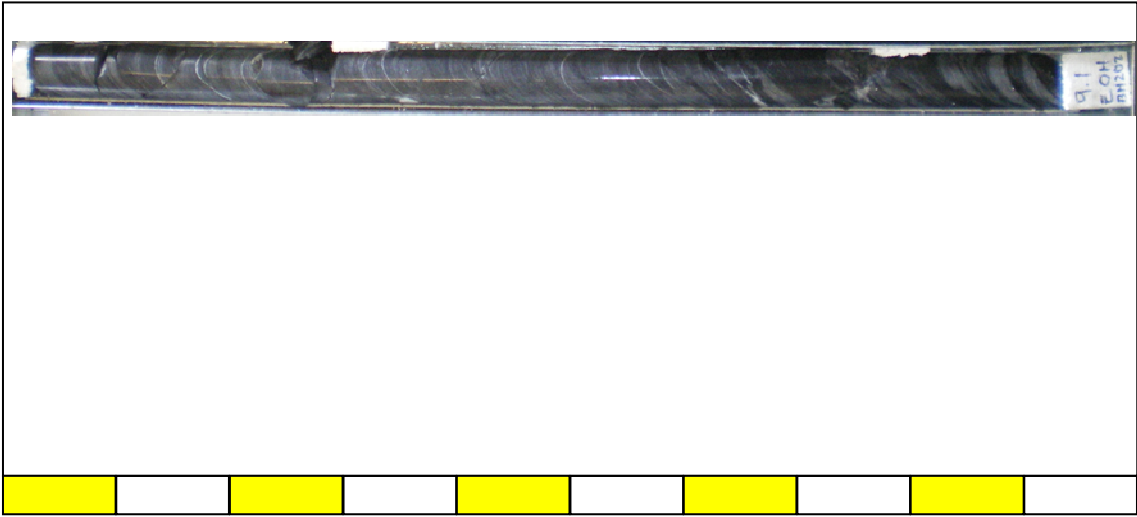
SMEC GOLD COAST BOREHOLE CORE LOG I:\PROJECTS\3003659\005_OPERATIONS\DD15 GEOTECHNICAL INVESTIGATIONS\GINT FILES\SMITH - OLSEN BOREHOLES (CURRENT).GPJ 18/11/11



Borehole Number		BH202	
Box	1	of	2
Depth	2.50m	to	8.10m
Project	Smith St & Olsen Av		
Number	3003659		
Client	QDTMR		



Borehole Number		BH202	
Box	2	of	2
Depth	8.10m	to	9.10m
Project	Smith St & Olsen Av		
Number	3003659		
Client	QDTMR		



NOTES RELATING TO GEOTECHNICAL REPORTS AND SITE INVESTIGATION LOGS

GEOTECHNICAL REPORTS AND SITE INVESTIGATION LOGS

Geotechnical reports/logs are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work, and is supplemented by knowledge of the local geology and experience of the range of properties that may exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report/log has been prepared for a specific purpose (e.g. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (e.g. a twenty-storey building). In such cases, the report/log and the sufficiency of the existing work should be reviewed by SMEC in the light of the new proposal.

Every care is taken with the report/log content; however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities
- The actions of contractors responding to commercial pressures

If these occur, SMEC would be pleased to resolve the matter through further investigation, analysis or advice.

UNFORESEEN CONDITIONS

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report/log, SMEC should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

SUBSURFACE INFORMATION

Logs of a borehole, recovered core, test pit, excavated face, or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling/observation spacing's and the ground conditions. It is not always possible or economic to obtain continuous high-quality data. It should also be recognised that the volume of material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long-term monitoring of groundwater levels may be required to adequately identify groundwater conditions.