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**BOREHOLE ENGINEERING LOG HOLE NO: CURVE 22 BH01 CLIENT: TRANSPORT AND MAIN ROADS** POSITION: E: 359398, N: 8137552 (56 MGA94) PAGE: 1 OF 2 PROJECT: BLACK SPOT PROJECT SURFACE ELEVATION: 112.3 (AHD) DATE DRILLED: 7/8/13 TO 7/8/13 LOGGED BY: JP JOB NO: CB24735.04 DIP / AZIMUTH : 90° CHECKED BY: AJ LOCATION: KENNEDY HWY (CAIRNS - MAREEBA) DRILLING **MATERIAL** WATER LES & TESTS Œ CONSISTENCY MATERIAL DESCRIPTION MOISTURE  $\widehat{\Xi}$ DRILLING PENETRAT STRUCTURE **3RAPHIC** & CASING DEPTH Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components GROUND V EVELS & Other Observations WATER SAMPL FIELD 1 Ζ 90 VH ASPHALT: 0.10m. SILTY GRAVEL (GM): Brown, grey brown, fine to 0.20m 0.20: Moisture Content (%) = 3.8, Liquid Limit (%) = 25, Plastic Limit (%) = 17, Plasticity Index (%) = 8, Linear Shrinkage (%) = 1.5, % Passing 37.5mm: 100, % Passing 2.36mm: 53, % Passing 0.425mm: 36, % Passing 0.075mm: 25, % Passing 0.002mm: 4, Max. Dry Density (t/m³) = 2.1, OMC (%) = 8.5 medium gravel, angular, with some fine to coarse grained sand F Ϋ́ D From 0.70 m to 1.00 m with cobbles Н 111.3 -1.0 GM 1.45m 2.00m 110.3 -2.0 From 2.00 m trace of rootlets 2.50m SPT 3, 6, 7 N=13 SILTY GRAVEL (GM): Mottled brown, grey brown, red brown, fine to medium gravel, sub-angular, comprising high to very high strength quartzite, fine to coarse COLLUVIUM? OBSERVED grained sand, with fine to coarse grained sand. 2.95m 109.3 -3.0 NOT WB 108.3 4.0 RESIDUAL SOIL SILTY GRAVEL (GM): Brown, red brown, fine to medium gravel, angular, comprising low to high strength quartzite, fine grained sand, with fine to REDIDUAL SUIL
4.00: Moisture Content (%) = 13.7, Liquid Limit
(%) = 27, Plastic Limit (%) = 20, Plasticity Index
(%) = 7, Linear Shrinkage (%) = 5, % Passing
37.5mm: 100, % Passing 2.36mm: 66, %
Passing 0.425mm: 53, % Passing 0.075mm: 38,
% Passing 0.002mm: 7 02/12/2013 15:35 Н MD coarse grained sand. 4.45m <<DrawingFile>> -5.0 107.3 GM CB24735.04.GPJ 5.50m SPT 14, 14, 12 N=26 BOREHOLE Log CURRENT.GLB DRILLING SAMPLES & FIELD TESTS CONSISTENCY (Su) {N-value} DENSITY (SPT N-value) Hand Auger Disturbed Sample SPT Standard Penetration Test RR Rock Rolling VL Very Loose 0 - 4 VS Very Soft < 12 kPa {0-2} Env Soil Sample Undisturbed Tube Sample Auger Screw AT Auger Drill TC-bit HQ Air Track HQ Coring AS ı Loose 4 - 10 S Soft 12 - 25 {2-4} W Water Sample EW Env Water Sample **IBRARY** MD Medium Dense 10 - 30 F 25 - 50 {4-8} Auger Drill V-bit Washbore NQ NQ Coring NMLC NMLC Coring AD/V WB D Dense 30 - 50 St Stiff 50 - 100 {8-15} HP Hand Penetrometer HV Hand Vane Shear MOISTURE CONDITION
D = Dry M = Moist W = Wet DRILLING PENETRATION VD Very Dense 50 - 100 VSt Very Stiff 100 - 200 {15-30} OFFICE F Firm H Hard VE Very Easy E Easy VH Very Hard (P: Peak Su R: Residual Su) CO Compact >50/150mm Hard > 200 kPa {>30} N SPT blows per 300mm HW SPT penetration by hammer weight **GROUNDWATER SYMBOLS** RW SPT penetration by rod weight = Water level (static) = Water level (during drilling)

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## **BOREHOLE ENGINEERING LOG HOLE NO: CURVE 22 BH01** CLIENT: TRANSPORT AND MAIN ROADS POSITION: E: 359398, N: 8137552 (56 MGA94) PAGE: 2 OF 2 PROJECT: BLACK SPOT PROJECT SURFACE ELEVATION: 112.3 (AHD) DATE DRILLED: 7/8/13 TO 7/8/13 LOGGED BY: JP JOB NO: CB24735.04 DIP / AZIMUTH : 90° CHECKED BY: AJ LOCATION: KENNEDY HWY (CAIRNS - MAREEBA) DRILLING **MATERIAL** LES & TESTS Œ CONSISTENCY MATERIAL DESCRIPTION MOISTURE $\widehat{\Xi}$ STRUCTURE GRAPHIC DEPTH CASING Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components DRILLING GROUND V EVELS & Other Observations Ζ 90 SILTY GRAVEL (GM): Brown, red brown, fine to medium gravel, angular, comprising low to high strength quartzite, fine grained sand, with fine to RESIDUAL SOIL RESIDUAL SOIL Н GM coarse grained sand. (continued) EXTREMELY TO HIGHLY WEATHERED ROCK GNEISS: Orange brown, pale brown, extremely weathered, extremely low strength, appears as SANDY GRAVEL (GM), fine to medium grained angular gravel, fine to coarse grained sand, with fines. 7.00m SPT 9, 8, 6 N=14 105.3 7.00: Moisture Content (%) = 12.5, Liquid Limit (%) = 26, Plastic Limit (%) = 19, Plasticity Index (%) = 7, Linear Shrinkage (%) = 5, % Passing 2.36mm: 88, % Passing 0.425mm: 49, % Passing 0.075mm: 34, % Passing 0.002mm: 7 VH 7.45m -8.0 GNEISS: Orange brown, extremely weathered, extremely low to very low strength, appears as SANDY SILT (ML), low plasticity, fine to medium EXTREMELY WEATHERED ROCK grained sand, with fine to medium grained angular gravel. OBSERVED 8.50m SPT 2, 4, 4 N=8 F MD MB NOT 8.95m 103.3 -9.0 9.10m GNEISS: Grey brown, grey red brown, extremely to highly weathered, very low to low strength, appears as GRAVELLY SAND (GM), fine to medium grained angular gravel, fine to medium grained sand, trace of fines, fine to coarse grained sand, fine to medium grained angular gravel, with fines (Phyllite). EXTREMELY TO HIGHLY WEATHERED ROCK Н F D Н SPT 5, 16, 21 N=37 102.3 -10.0 02/12/2013 HIGHLY WEATHERED ROCK GNEISS: Grey, dark grey, orange brown, highly weathered, low to medium strength. 10.45m <<DrawingFile>> VΗ 101.3 CB24735.04.GPJ Terminated Drill Hole at 11.25 m refusal on estimated medium to high strength material BOREHOLE Log CURRENT.GLB DRILLING SAMPLES & FIELD TESTS CONSISTENCY (Su) {N-value} DENSITY (SPT N-value) Hand Auger Disturbed Sample SPT Standard Penetration Test RR Rock Rolling VL Very Loose 0 - 4 VS < 12 kPa {0-2} Env Soil Sample Undisturbed Tube Sample Auger Screw AT Auger Drill TC-bit HQ Air Track HQ Coring AS Loose 4 - 10 S Soft 12 - 25 {2-4} Water Sample EW Env Water Sample **IBRARY** MD Medium Dense 10 - 30 F 25 - 50 {4-8} Firm Auger Drill V-bit Washbore NQ NQ Coring NMLC NMLC Coring AD/V WB D Dense 30 - 50 St Stiff 50 - 100 {8-15} HP Hand Penetrometer HV Hand Vane Shear MOISTURE CONDITION D = Dry M = Moist W = Wet DRILLING PENETRATION VD Very Dense 50 - 100 VSt Very Stiff 100 - 200 {15-30} OFFICE F Firm H Hard VE Very Easy E Easy VH Very Hard (P: Peak Su R: Residual Su) CO Compact >50/150mm Hard > 200 kPa {>30} N SPT blows per 300mm HW SPT penetration by hammer weight **GROUNDWATER SYMBOLS** RW SPT penetration by rod weight = Water level (static) = Water level (during drilling)

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